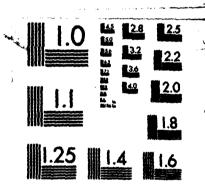
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COASTAL BASIN
SAUGUS, MASSACHUSETTS
LYNN, MASSACHUSETTS

WALDEN POND OUTLET DAM

MA 00246

**WALDEN POND EAST END DAM** 

MA 00235

PHASE I INSPECTION REPORT NATIONAL DAM INSPECTION PROGRAM



DEPARTMENT OF THE ARMY
NEW ENGLAND DIVISION, CORPS OF ENGINEERS
WALTHAM, MASSACHUSETTS 02154

**AUGUST 1978** 

#### REPRODUCED AT GOVERNMENT EXPENSE

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Cover program reads: Phase I Inspection Report, National Dam Inspection Program; however, the official title of the program is: National Program for Inspection of Non-Federal Dams; use cover date for date of report.

19. KEY WORDS (Continue on reverse side if necessary and identify by block number)

DAMS, INSPECTION, DAM SAFETY,

Coastal Basin Saugus, Massachusetts Lynn, Massachusetts Penny Brook

20. ABSTRACT (Continue on reverse side if necessary and identify by block number)

Both dams are earthfill with concrete fore. \_The Walden Pond east DAm is about 180 ft. long and about 12 ft. high. The other contains a 6 ft high by 30 ft long ungated spillway near its north end. The Walden Pond Outlet Dam is considered to be inpoor condition. The situation on the East End Dam is not as sever but is considered to be in fair condition. Both are intermediate in size and classified as having a high hazard potential. Additional investigations or major modifications are not necessary.

WALDEN POND OUTLET DAM
MA 00246

WALDEN POND EAST END DAM MA 00235

COASTAL BASIN SAUGUS, MASSACHUSETTS LYNN, MASSACHUSETTS

PHASE I INSPECTION REPORT NATIONAL DAM INSPECTION PROGRAM

# NATIONAL DAM INSPECTION PROGRAM

# PHASE I INSPECTION REPORT

Identification No.:

MA 00246

Name of Dam:

Walden Pond Outlet

Town:

Saugus, Massachusetts

County and State:

Essex County, Massachusetts

Stream:

Penny Brook

Date of Inspection:

July 6, 1978

\* \* \* \* \* \* \* \* \* \* \* \* \* \*

Identification No.:

MA 00235

Name of Dam:

Walden Pond East End

Town:

Lynn, Massachusetts

County and State:

Essex County, Massachusetts

Stream:

None

Date of Inspection:

July 7, 1978

#### BRIEF ASSESSMENT

The Walden Pond Outlet Dam was originally constructed in 1890. It was raised in 1905 when the Walden Pond East End Dam was constructed. Both dams are earthfill with concrete core. The former is 2,190 feet long and 42 feet high; the latter 180 feet long and about 12 feet high. The Walden Pond Outlet Dam contains a 6-foot high by 30-foot long ungated spillway near its north end. The reservoir is part of the City of Lynn water supply system. Walden Pond receives water from Hawkes Pond and discharges water to both Birch and Breed Ponds.

Owing primarily to severe erosion caused by motorbikes, and the partial exposure of the concrete core wall, the Walden Pond Outlet Dam must be

considered in poor condition. The situation on the East End Dam is not as severe and this dam can be considered to be in fair condition. The spillway structure is overgrown and contains debris.

The impounded volume of the reservoir, 4,100 acre feet, places the dams in the intermediate size classification. The dams are both in the high hazard potential category and thus hydraulically analyzed using the full probable maximum flood.

Reservoir storage will reduce the maximum probable discharge of 1,411 cfs to a test flood of 642 cfs. As the spillway can discharge this test flood with approximately 2 feet of freeboard remaining, the chance of the dam overtopping is considered minimal.

A failure of the Walden Pond Outlet Dam could produce a flow as high as 184,000 cfs; a failure of the East End Dam a flow of 4,200 cfs. Failure flows of these magnitudes would cause destruction of dwellings and other property in the watercourses below the dams and would endanger human life.

Additional investigations or major modifications are not necessary. Remedial measures that should be implemented by the owner within one year after receipt of this Phase I Inspection Report are described in Section 7. Although the dam is in no danger of overtopping, and the crest width is larger than the height of the dam, the erosion on the downstream slope and the exposure of the core can only increase. These areas should be repaired and every effort made to keep the situation from again deteriorating.

The owner should institute a regular program of inspection and maintenance and should develop a flood warning system.

Gustav A. Diezemann, P. E. New York State Lic. 027062

This Phase I Inspection Report on the Walden Pond Outlet Dam and the Walden Pond East End Dam has been reviewed by the undersigned Review Board members. In our opinion, the reported findings, conclusions, and recommendations are consistent with the Recommended Guidelines for Safety Inspection of Dams, and with good engineering judgment and practice, and hereby submitted for approval.

CHARLES G. TIERSCH, Chairman Chief, Foundation and Materials Branch Engineering Division

FRED J. RAVENS, Jr., Member Chief, Design Branch Engineering Division

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SAUL COOPER, Member Chief, Water Control Branch Engineering Division

APPROVAL RECOMMENDED:

JOE B. FRYAR Chief, Engineering Division

#### **PREFACE**

This report is prepared under guidance contained in the Recommended Guidelines for Safety Inspection of Dams, for Phase I Investigations. Copies of these guidelines may be obtained from the Office of Chief of Engineers, Washington, D.C. 20314. The purpose of a Phase I Investigation is to identify expeditiously those dams which may pose hazards to human life or property. The assessment of the general condition of the dam is based upon available data and visual inspections. Detailed investigation and analyses involving topographic mapping, subsurface investigations, testing, and detailed computational evaluations are beyond the scope of a Phase I investigation; however, the investigation is intended to identify any need for such studies.

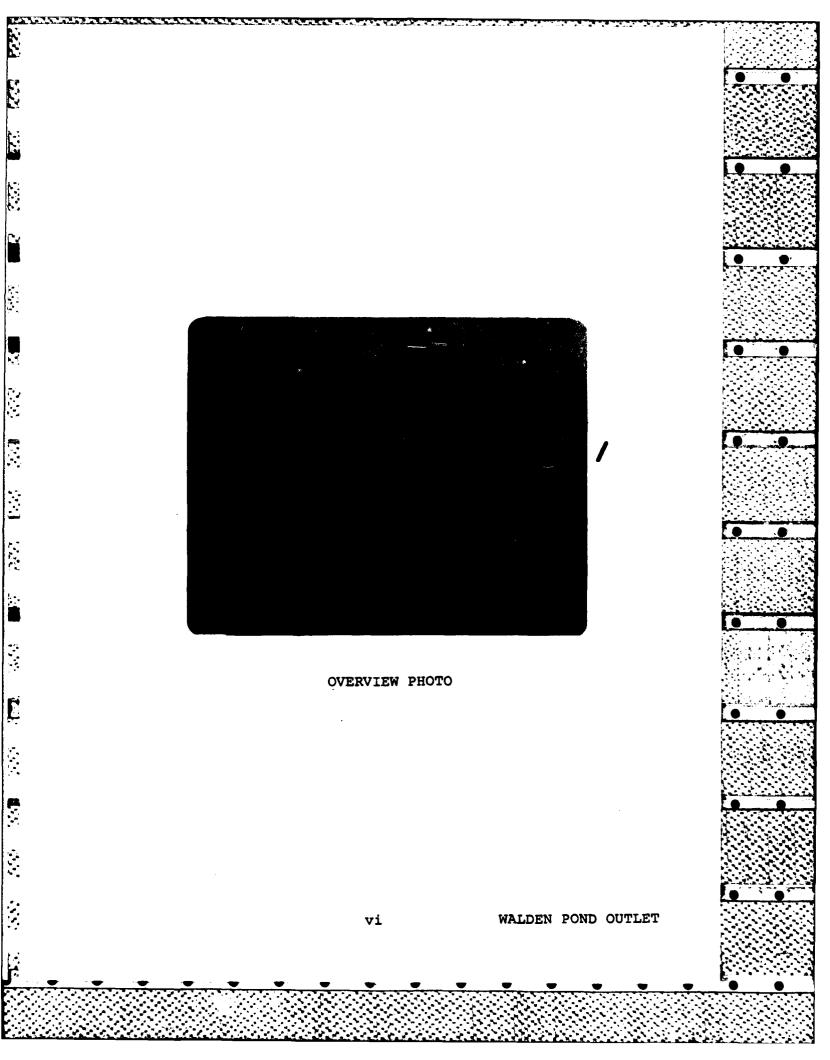
In reviewing this report, it should be realized that the reported condition of the dam is based on observations of field conditions at the time of inspection, along with data available to the inspection team. In cases where the reservoir was lowered or drained prior to inspection, such action, while improving the stability and safety of the dam, removes the normal load on the structure and may obscure certain conditions which might otherwise be detectable if inspected under the normal operating environment of the structure.

It is important to note that the condition of a dam depends on numerous and constantly changing internal and external conditions, and is evolutionary in nature. It would be incorrect to assume that the present condition of the dam will continue to represent the condition of the dam at some point in the future. Only through continued care and inspection can there be any chance that unsafe conditions be detected.

Phase I inspections are not intended to provide detailed hydrologic and hydraulic analyses. In accordance with the established Guidelines, the Spillway Test flood is based on the estimated "Probable Maximum Flood" for the region (greatest reasonably possible storm runoff), or fractions thereof. Because of the magnitude and rarity of such a storm event, a finding that a spillway will not pass the test flood should not be interpreted as necessarily posing a highly inadequate condition. The test flood provides a measure of relative spillway capacity and serves as an aid in determining the need for more detailed hydrologic and hydraulic studies, considering the size of the dam, its general condition and the downstream damage potential.

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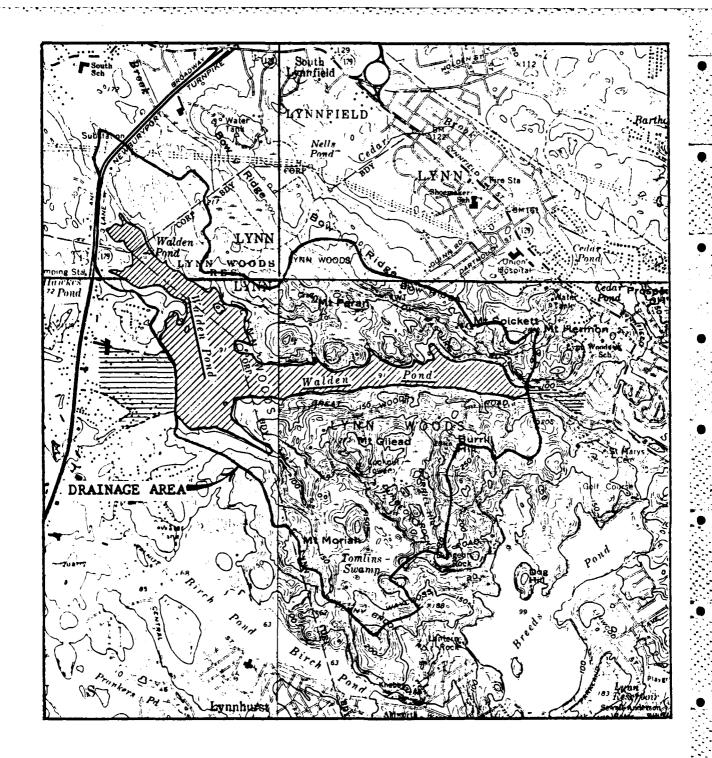




OVERVIEW PHOTO

vi

WALDEN POND EAST END



WALDEN POND OUTLET AND WALDEN POND EAST

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#### PHASE I INSPECTION REPORT

#### WALDEN POND OUTLET

#### WALDEN POND EAST END DAM

#### SECTION I

#### PROJECT INFORMATION

### 1.1 General

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a. <u>Authority.</u> Public Law 92-367, August 8, 1972, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a National Program of Dam Inspection throughout the United States. The New England Division of the Corps of Engineers has been assigned the responsibility of supervising the inspection of dams within the New England Region. Chas. T. Main, Inc. has been retained by the New England Division to inspect and report on selected dams in the State of Massachusetts. Authorization and notice to proceed were issued to Chas. T. Main, Inc. under a letter of May 3, 1978, from Ralph T. Garver, Colonel, Corps of Engineers. Contract No. DACW33-78-D328 has been assigned by the Corps of Engineers for this work.

## b. Purpose.

- (1) Perform technical inspection and evaluation of non-Federal dams to identify conditions which threaten the public safety and thus permit correction in a timely manner by non-Federal interests.
- (2) Encourage and prepare the states to initiate quickly effective dam safety programs for non-Federal dams.
- (3) To update, verify and complete the National Inventory of Dams.

#### 1.2 Description of Project

- a. <u>Location</u>. The Walden Pond Project is located in Essex County, Massachusetts. The Walden Pond Outlet is located in the Town of Saugus. The Walden Pond East End dam is in the City of Lynn.
- b. <u>Description of Project</u>. The Walden Pond Outlet dam was constructed in 1890 and raised in 1905 when the East End dam was constructed. The Outlet dam is about 2,190 feet long and 42 feet high; the East End dam is 180 feet long and appears to be about 12 feet high. The spillway at the Outlet, built in 1930, is 30 feet wide by 6 feet deep. Also at the Outlet

is a gate house controlling gravity feed to Birch Pond, and an emergency blowoff arrangement which can be directed either to Birch Pond or the watercourse below the dam. Walden Pond receives pumped water from Hawkes Pond. At the East End is a 16.5 mgd pumping station which pumps water to Breeds Pond. Under certain water level conditions, this 36-inch line flows by gravity.

- c. <u>Size Classification</u>. Owing to the impoundment of approximately 4,100 acre feet below the crest, the dams fall within the intermediate size classification.
- d. <u>Hazard Classification</u>. The areas downstream of the dams which would be endangered if the dams failed are urban or, at best, suburban in nature. The dams are considered to have high hazard classification.
  - e. Ownership. The dams are owned by the City of Lynn.
- f. Operator. Mr. Patrick McGrath, Superintendent of Water, Department of Public Works, Lynn, Massachusetts, (617) 592-7900, Ext. 242.
- g. Purpose of Dams. The reservoir impounded by the dams is a part of the City of Lynn's water supply system.
  - h. <u>Design and Construction History</u>. Other than a drawing, excerpts which are a part of this report, nothing is known of the design and construction history of this project.
  - i. <u>Normal Operating Procedures</u>. The water level is normally kept below the spillway level by means of releases to Birch Pond. Inflows exceeding outflow and storage capabilities would discharge through the spillway.

### 1.3 Pertinent Data

a. <u>Drainage Area</u>. Walden Pond has an approximate 1.75 square mile drainage area of semi-wooded, hilly land.

### b. Discharge at Damsite.

- (1) The outlet works consist of a 36-inch gravity line to Birch Pond and an emergency blowoff arrangement which can be directed either to Birch Pond or Hawkes Brook.
  - (2) The maximum known flood at the site is unknown.
- (3) The ungated spillway capacity exceeds the test flood of 642 cfs.
  - (4) There is no gated spillway capacity.
  - (5) There is no gated spillway capacity.

			,•	
of 642 cfs.	(6)	The total spillway capacity exceed	is the test flood	
c.	Eleva	ation (Feet Above MSL)		•
	(1)	Top of dam	E1. 97 ± (both dams)	
	(2)	Maximum design surcharge	E1. 95 ± (both dams)	
	(3)	Full flood control pool	n/a	
	(4)	Recreation pool	n/a	
	(5)	Spillway crest (ungated)	E1. 91 + (Outlet dam)	
	(6)	Upstream portal invert diversion	tunnel N/A	•
	(7)	Streambed at centerline of dam	E1. 55 <sup>+</sup> (Outlet dam) E1. 85 <sup>+</sup> (East End dam)	
	(8)	Maximum tailwater	N/A	
d.	Reser	rvoir (Feet)		
	(1)	Length of maximum pool	8,000 ±	
	(2)	Length of recreation pool	n/A	
	(3)	Length of flood control pool	N/A	
e.	Stora	age (Acre-Feet)		
	(1)	Recreation pool	4,100 ± (at crest)	
	(2)	Flood control pool	N/A	
	(3)	Design surcharge	5,100 ±	
	(4)	Top of dam	5,500 ±	
f.	Reser	rvoir Surface (Acres)		
	(1)	Top of dam	243 ±	
	(2)	Maximum pool	239 ±	
	(3)	Flood control pool	n/A	
	(4)	Recreation pool	N/A	
	(5)	Spillway crest	230	
				្តី

g.	Dam		
	(1)	Туре	Earthfill with concrete core
	(2)	Length	Outlet - 2,190 feet East End - 180 feet
	(3)	Height	Outlet - 42 feet East End - 12 feet
	(4)	Top Width	Outlet - 52 feet East End - 8 feet
	(5)	Side Slope	Outlet - 1-3/4:1 Upstream, Varies downstream
			East End - Unknown
	(6)	Zoning	Unknown
	(7)	Impervious Core	Unknown
	(8)	Cutoff	Unknown
	(9)	Grout Curtain	Unknown
	(10)	Other	N/A
h.	<u>Sp111</u>	way	
	(1)	Туре	Ungated weir
	(2)	Length of Weir	30 feet
	(3)	Crest Elevation	E1. 91 ±
	(4)	Gates	None
	(5)	U/S Channel	N/A
	(6)	D/S Channel	N/A

i. Regulating Outlets. The outlet works consist of a 36-inch gravity flow line to Birch Pond and an emergency blowoff arrangement which can be directed to either Birch Pond or Hawkes Brook. The elevations of these lines are not known.

(7)

General

N/A

#### **ENGINEERING DATA**

# 2.1 Design

There are some drawings of Walden Pond Outlet Dam available at the Town of Lynn City Hall (Room 401) showing the dam cross sections. Other than these drawings, which are made part of this report, there are no design data nor records available.

# 2.2 Construction

The Walden Pond Outlet dam was built in 1890 and raised in 1905. The spillway was built in 1930. The Walden Pond East dam was built in 1905. There are no detailed construction records available.

# 2.3 Operation

Some flow data are kept but are not relevant to this investigation.

# 2.4 Evaluation

- a. Availability. Other than the drawings mentioned above, there are no engineering data available.
- b. Adequacy. The lack of in-depth engineering data does not allow for a definitive review. Therefore, the adequacy of these dams, structurally and hydraulically, cannot be assessed from the standpoint of review of design calculations, but must be based primarily on the visual inspection, past performance history, and sound hydrologic and hydraulic engineering judgment.
  - c. <u>Validity</u>. N/A

### VISUAL INSPECTION

# 3.1 Findings

- a. General. The Phase I visual inspections of the Walden Pond Outlet and East End Dams were conducted on July 6, 1978. The project is part of the water supply of the City of Lynn, releases being possible to Birch and Breeds Pond. The original Outlet Pond Dam is almost 90 years old, and was raised almost 75 years ago when the East End Dam was constructed. The projects are unprotected and obviously misused by the public.
- b. <u>Dams</u>. Both dams are earthfill with concrete core. The downstream face of the core wall of the Outlet dam is exposed in several places due to severe erosion caused by motorbiking. While there is evidence of motorbiking on the East End Dam, the erosion is not near as severe. There is motorbiking on the crests of the dam but the situation does not appear serious.

The Outlet dam is heavily overgrown on both the up and downstream slopes. The riprap on the downstream slope is somewhat displaced in many areas but there are no obvious signs of major movement. There is some growth other than grass on the up and downstream slope of the East End dam. There is evidence that the lower portion of this low dam was riprapped at one time, however much of it has sloughed.

There are no significant misalignments of either dam, however both dams obviously suffer from lack of maintenance and supervision. The Outlet dam, owing mainly to the exposure of the core wall, must be considered to be in poor condition. The East End dam is in fair condition.

- c. Appurtenant Structures. Both brick and concrete gatehouses, and the concrete control structure below the Outlet dam, are, apart from minor spalling, in good condition. The service bridges require painting. The stone-lined spillway at the Outlet dam is overgrown, contains debris and is in generally poor condition.
- d. Reservoir Area. The banks surrounding the reservoir are hilly and wooded. There are no houses along the perimeter of the reservoir. There appears to be little or no possibility of landslides or conditions which might result in a sudden increase of sediment load in the reservoir.

e. <u>Downstream Channels</u>. In the case of the Outlet dam, the channel immediately downstream of the spillway is overgrown. Beyond that is a broad, semi-marsh. There are several homes and commercial and industrial buildings in the general watercourse below the dam, both before and after its intersection with U.S. Route 1. The exact number and identity of the structures affected would depend upon the magnitude and location of the breach.

Should a failure of the East End occur, water would flow through a thickly settled residential area.

## 3.2 Evaluation

The visual inspections during site examination indicate that both the Outlet and East End dams have been neglected with respect to maintenance. The Outlet dam and spillway must be considered to be in poor condition, and the East End dam to be in fair condition. The gatehouses are operable, obviously, and in good condition. The reservoir itself is not a factor in evaluating the dams. The watercourses below both dams are inhabited to the extent that property and life would be in jeopardy if the dam(s) failed.

#### **OPERATIONAL PROCEDURES**

# 4.1 Procedures

Walden Pond receives water from Hawkes Pond and the Ipswich River. Water level is maintained by gravity feed to Birch Pond and pumping to Breeds Pond. There is an emergency blowoff arrangement which can divert water either to Birch Pond or Hawkes Brook. Inflows in excess of these capabilities would flow over the spillway.

# 4.2 Maintenance of Dam

There appear to be no definite maintenance procedures of the dam in effect.

# 4.3 Maintenance of Operating Facilities

The gates controlling the outflows are maintained on a yearly basis, according to the owner.

# 4.4 Warning System

There is no warning system.

# 4.5 Evaluation

Apart from the daily operation to meet the water supply demands, the operational procedures are minimal. Maintenance of the dam and spillway could be improved. Recommendations for improving this situation are given in Section 7.3.

#### HYDRAULIC/HYDROLOGIC

# 5.1 Evaluation of Features

a. <u>Design Data</u>. The hydraulic/hydrologic analysis was made in accordance with "Preliminary Guidance for Estimating Maximum Probable Discharges in Phase I Dam Safety Investigations", "Estimating Effect of Surcharge Storage on Maximum Probable Discharges", and "Rule of Thumb Guidance for Estimating Downstream Dam Failure Hydrographs" as furnished by the New England Division, Corps of Engineers and "Recommended Guidelines for Safety Inspection of Dams" as issued by the Department of the Army, Office of the Chief of Engineers.

U.S.G.S. Quadrangle maps were used to determine reservoir and drainage areas. Where practicable, spillway dimensions were obtained by direct measurement. Hydraulic coefficients were assigned on the basis of experience and engineering judgment.

- b. Experience Data. No specific experience data with respect to the hydraulic/hydrological characteristics of the project are known to exist.
- c. <u>Visual Observations</u>. Spillway is totally overgrown. The stone-lined channel is filled with debris. There is an emergency blowoff directional control in the low concrete structure below dam. Manhole covers are secured. Riprap on the front face is in good condition.
- d. Overtopping Potential. A Probable Maximum Flood (PMF) of 1,411 cfs was determined. Owing to its intermediate size and high hazard classifications, the PMF was used to determine the Peak Outflow (or test flood) of 642 cfs. The spillway can discharge this flow with about 2 feet of freeboard remaining. Thus, the dams are not in danger of overtopping.

Assuming a 525 foot breach in the Outlet dam, a Peak Failure Flood of about 184,000 cfs was determined. Such a discharge would destroy several dwellings on the banks of the watercourse below the dam, U. S. Route 1 would be in danger of being flooded, and human life would be endangered.

Assuming a 60 foot breach in the East End dam would result in a Peak Failure Outflow of 4,200 cfs. This flow would run through a recreational area and then through a residential area before discharging into Sluice Pond which, in turn, would overflow its banks. There would be flooding, destruction of property, and possible loss of human life in such an event.

The areas of impact immediately downstream of both dams are shown on the location map.

#### STRUCTURAL STABILITY

# 6.1 Evaluation of Structural Stability

- a. <u>Visual Observations</u>. Nothing was noted which would indicate that the dams were unstable.
- b. <u>Design and Construction Data</u>. No design nor construction data are available other than a cross section of the dam.
  - c. Operating Records. Not applicable.
- d. <u>Post Construction Changes</u>. No data concerning any post construction changes are available.
- e. <u>Seismic Stability</u>. The dam is located in Seismic Zone 3. Because of its configuration and condition and the low head of water retained, a seismic anlyasis is not considered warranted.

# ASSESSMENT, RECOMMENDATIONS AND REMEDIAL MEASURES

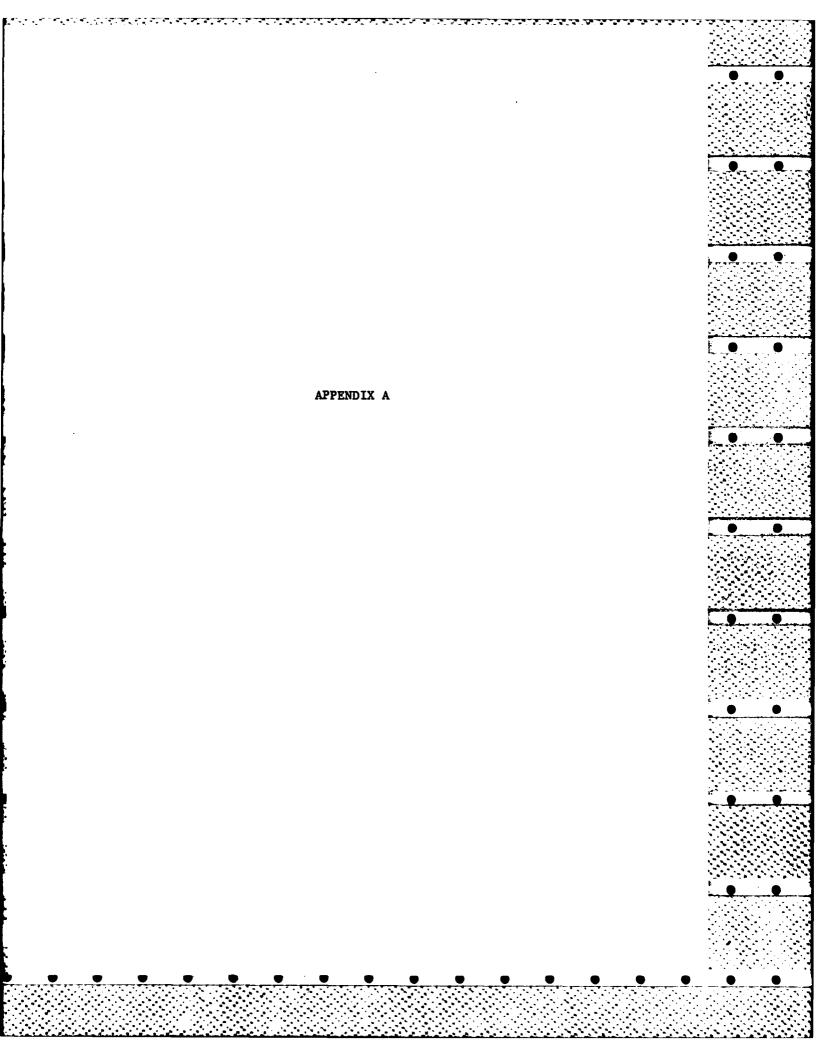
## 7.1 Dam Assessment

- a. <u>Condition</u>. The condition of the Outlet dam is poor; that of the East End dam is fair.
- b. Adequacy of Information. The lack of in-depth engineering data did not allow for a definitive review. Therefore, the adequacy of the dams could not be assessed from the standpoint of reviewing design and construction data, but is based primarily on visual inspection, past performance history and engineering judgment.
- c. <u>Urgency</u>. The required repair and maintenance work should be accomplished within one year of the receipt of this report by the owner.
- d. <u>Need for Additional Investigation</u>. There is no need for additional investigation.
- 7.2 <u>Recommendations</u>. Additional engineering investigations or major modifications to the dams are not required.

# 7.3 Remedial Measures

- a. Alternatives. Not applicable.
- b. Operating and Maintenance Procedures.
  - (1) All eroded areas should be filled and seeded.
- (2) The spillway should be cleaned of growth and debris and repairs made if required.
- (3) The owner of the dam should develop and implement procedures which would include annual inspection of the dams and the initiation of repairs. This should include repair of all spalled concrete and repair and painting of the service bridge and gatehouse as required.
  - (4) Motorbiking on the dam should be stopped.

- (5) Around the clock surveillance should be provided by the owner during periods of unusually heavy precipitation.
- (6) The owner should develop a formal warning system with local officials for alerting downstream residents in case of emergency.



## VISUAL INSPECTION CHECK LIST PARTY ORGANIZATION

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PROJECT WALDEN POND OUTLET	DATE JULY6, A78
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none	
no paving	
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heavy growth & trees on U/s slope	
20' of exposed core wall on	
1/5 slope	
none	
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	CHECK LIST	
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ROJECT FEATURE	NAME	•
AREA EVALUATED	CONDITION	
UTLET WORKS - INTAKE CHANNEL AND INTAKE STRUCTURE		
. Approach Channel		
Slope Conditions Bottom Conditions		
Rock Slides or Falls		
Log Boom	NOT	
Debris	APPLICABLE	•
Condition of Concrete Lining	APPEICABLE	
Drains or Weep Holes		
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Condition of Concrete		
Stop Logs and Slots	·	
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INSPECTION O		
PROJECT WALDEN POND DUTLET	DATE JULY 6, 1978	
PROJECT FEATURE	NAME	
AREA EVALUATED	CONDITION	
OUTLET WORKS - TRANSITION AND CONDUIT		
General Condition of Concrete		
Rust or Staining on Concrete		
Spalling		
Erosion or Cavitation		
Cracking	Not	
Alignment of Monoliths	APPLICABLE	
Alignment of Joints		
Numbering of Monoliths		
	•	
·		
·		
	•	
		5

### INSPECTION CHECK LIST

PROJECT FEATURE	NAME
AREA EVALUATED	CONDITION
OUTLET WORKS - SPILLWAY WEIR, APPROACH AND DISCHARGE CHANNELS	
a. Approach Channel  General Condition	
Loose Rock Overhanging Channel	
Trees Overhanging Channel Floor of Approach Channel	
b. Weir and Training Walls	
General Condition of Concrete	poor
Rust or Staining	50me
Spalling	some
Any Visible Reinforcing	none
Any Seepage or Efflorescence	none
Drain Holes	none
c. Discharge Channel	
General Condition	
Loose Rock Overhanging Channel	1 il merarown
Trees Overhanging Channel	heavily overgrown with regetation
Floor of Channel	) pour iveg
Other Obstructions	
	6

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INSPECTION CH	HECK LIST	
PROJECT WALDEN POND OUNET	DATE JULY 6,1978	
PROJECT FEATURE	NAME	
AREA EVALUATED	CONDITION	
OUTLET WORKS - CONTROL TOWER		
a. Concrete and Structural	ı	
General Condition	1	
Condition of Joints	ı	
. Spalling	•	
Visible Reinforcing		
Rusting or Staining of Concrete	ı	
Any Seepage or Efflorescence		
Joint Alignment	NOT APPLICABLE	
Unusual Seepage or Leaks in Gate Chamber	APPLICAGE	•
Cracks		
Rusting or Corrosion of Steel		
b. Mechanical and Electrical		
Air Vents		
Float Wells		
Crane Hoist		•
Elevator		
Hydraulic System		
Service Gates		
Emergency Gates		
Lightning Protection System	·	
Emergency Power System		
Wiring and Lighting System		7

# INSPECTION CHECK LIST DATE JULY 6, 1978 PROJECT WALDEN POND BUTLET NAME PROJECT FEATURE CONDITION AREA EVALUATED OUTLET WORKS - OUTLET STRUCTURE AND OUTLET CHANNEL (bate House) General Condition of Concrete POOR some Rust or Staining some Spalling Erosion or Cavitation none Visible Reinforcing none Any Seepage or Efflorescence Condition at Joints O.K. Drain holes none Channel Loose Rock or Trees Overhanging Channel Condition of Discharge Channel

# INSPECTION CHECK LIST PROJECT WALDEN POND OUTLET DATE JULY 6,1978 PROJECT FEATURE \_ NAME AREA EVALUATED CONDITION OUTLET WORKS - SERVICE BRIDGE a. Super Structure 0.K. Bearings 0. K. Anchor Bolts 0.K. Bridge Seat O.K. Longitudinal Members Under Side of Deck O.K. Secondary Bracing 0, K. Deck none Drainage System In Need of Repair Railings **Expansion Joints** O.K. In Need of Paint Paint b. Abutment & Piers Poor General Condition of Concrete 0.K. Alignment of Abutment Approach to Bridge 0.K. Condition of Seat & Backwall

## VISUAL INSPECTION CHECK LIST PARTY ORGANIZATION

PROJECT Walden Pond East	DATE July 7, 1978  TIME 11:30AM  WEATHER WARM & SUNNY  W.S. ELEV. 91± U.S DN.S	
PARTY:  1. J. GOODRICH  2. D. FISCHER  3. 4.		
PROJECT FEATURE  1. 2.	INSPECTED BY REMARKS	
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INSPECTION (	CHECK LIST	
PROJECT WALDEN POND EAST	DATE JULY 7, 1978	
PROJECT FEATURE	NAME	
AREA EVALUATED	CONDITION	
DIKE EMBANKMENT	\$\$ 97±	
Crest Elevation	76.972	
Current Pool Elevation	9/	
Surface Cracks	none	
Pavement Condition	none no pavement none	
Movement of Settlement of Crest	nonl	
Lateral Movement	none	
Vertical Alignment	seeme ak.	
Horizontal Alignment	O.K.	
Condition at Abutment and at Concrete Structures	no concrete structures	•
Indications of Movement of Structural Items on Slopes	none	
Trespassing on Slopes	none	
Sloughing or Erosion of Slopes or Abutments	none	
Rock Slope Protection - Riprap Failures	none .	
Unusual Movement or Cracking at or near Toes	none	
Unusual Embankment or Downstream Seepage	none	
Piping or Boils	more	
Foundation Drainage Features	-	
Toe Drains	_	
<del>Inchiumente-on-Gyote</del> m		
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INSPECTION C	HECK LIST	
PROJECT WALDEN POND EAST	DATE JULY 7, 1978	
PROJECT FEATURE	NAME	
AREA EVALUATED	CONDITION	
CONCRETE DAM		
Concrete Surfaces		
Structural Cracking		
Movement Horizontal & Vertical Alignment		<b>I</b> • • • • • • • • • • • • • • • • • • •
Junctions	NoT	
Drains Foundation, Joint, Face	APPLICABLE	
Water Passages		
Seepage or Leakage		
Monolith Joints Construction Joints		
Foundation		
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·	•	
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INSPECTION	CHECK LIST	
PROJECT WALDEN POND EAST	DATE JULY 7, 1978	
PROJECT FEATURE	NAME	
AREA EVALUATED	CONDITION	
OUTLET WORKS - INTAKE CHANNEL AND INTAKE STRUCTURE		
a. Approach Channel		
Slope Conditions		
Bottom Conditions		
Rock Slides or Falls		
Log Boom	NoT	# 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
Debris	APPLICABLE	
Condition of Concrete Lining		
Drains or Weep Holes		
b. Intake Structure		
Condition of Concrete		
Stop Logs and Slots		
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INSPECTION C	JECV LICT		• •
PROJECT WALDEN POND EAST	DATE JULY 7,1978		
PROJECT FEATURE	NAME		
AREA EVALUATED	CONDITION		
·			
CUTLET WORKS - TRANSITION AND CONDUIT	·		
General Condition of Concrete			
Rust or Staining on Concrete			
Spalling			
Erosion or Cavitation	NoT		
Cracking	APPLICABLE		
Alignment of Monoliths			
Alignment of Joints			
Numbering of Monoliths			
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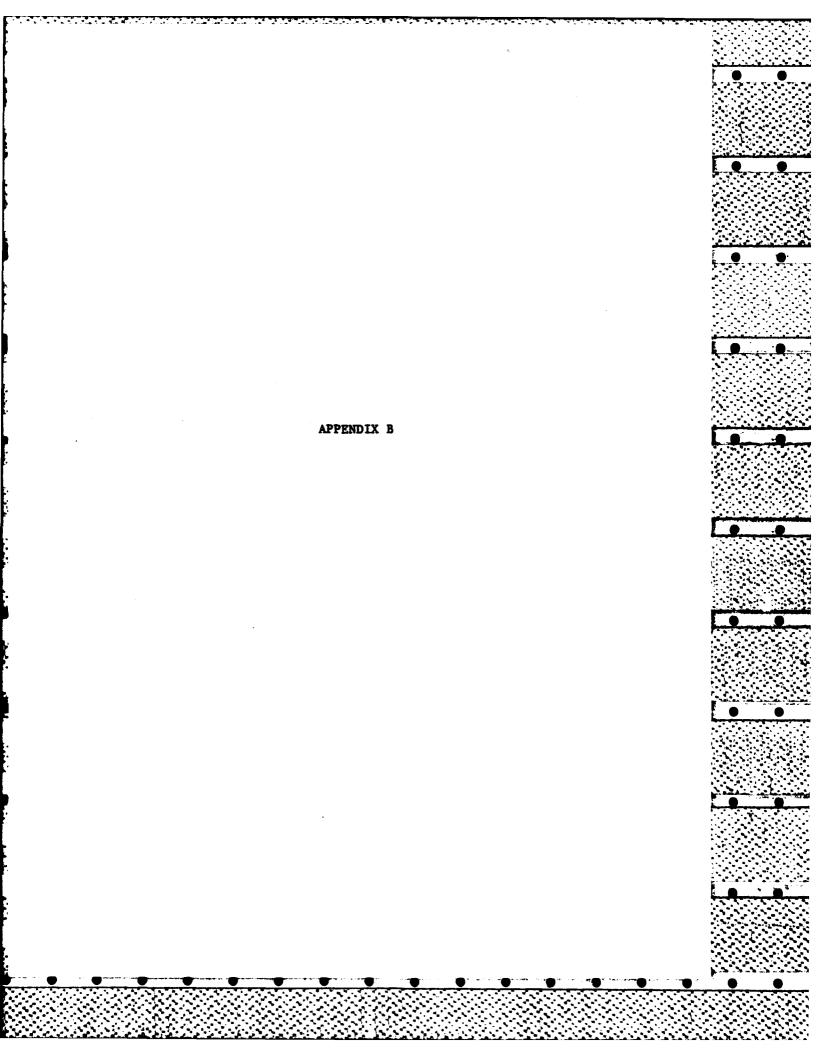
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INSPECTION CH		
PROJECT WALDEN POND EAST	DATE JULY 7,1978	
PROJECT FEATURE	NAME	
AREA EVALUATED	CONDITION	
OUTLET WORKS - SPILLWAY WEIR, APPROACH AND DISCHARGE CHANNELS		
a. Approach Channel		
General Condition		
Loose Rock Overhanging Channel		
Trees Overhanging Channel		
Floor of Approach Channel		
o. Weir and Training Walls	NoT	
General Condition of Concrete	APPLICABLE	
Rust or Staining		
Spalling		
Any Visible Reinforcing		
Any Seepage or Efflorescence		
Drain Holes	·	
c. Discharge Channel		
General Condition		
Loose Rock Overhanging Channel		
Trees Overhanging Channel	•	
Floor of Channel		
Other Obstructions		
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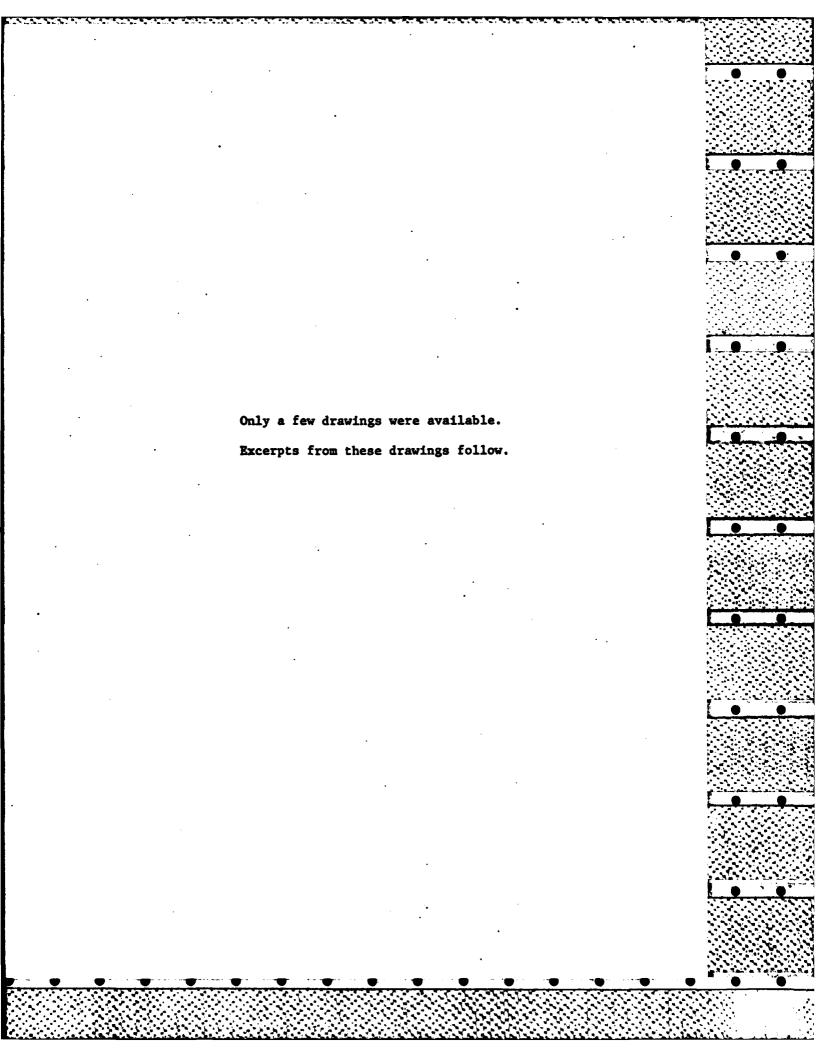
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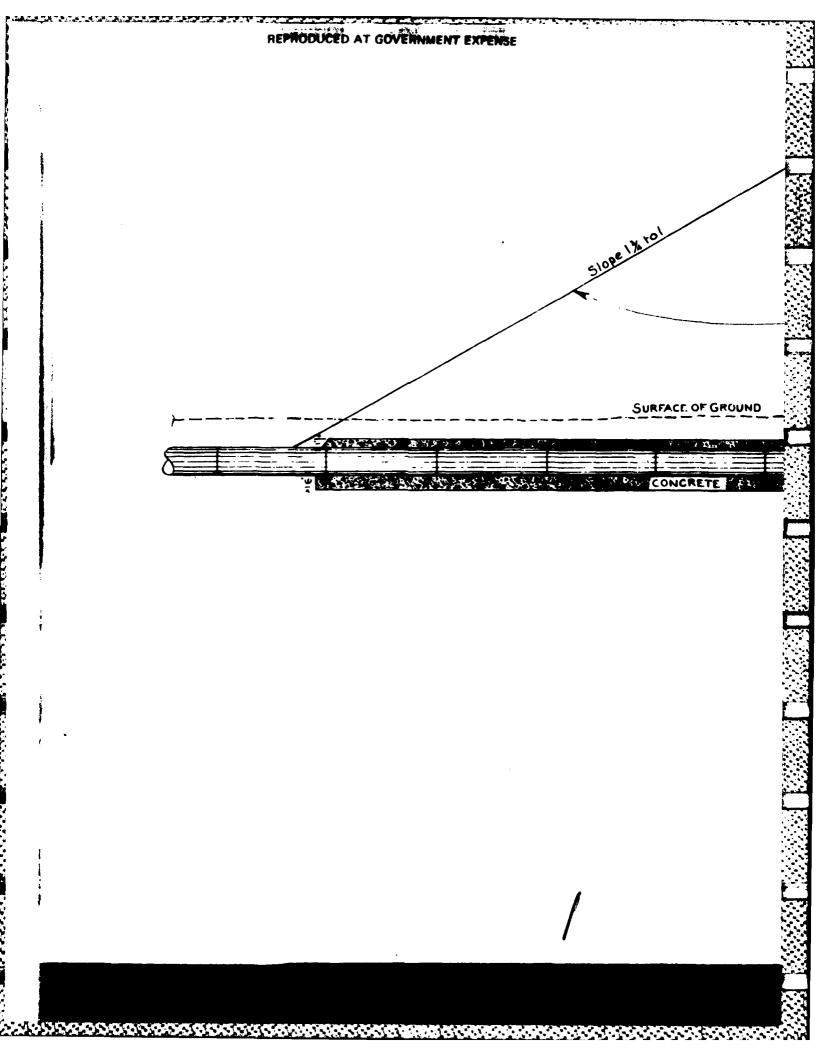
INSPECTION PROJECT WALDEN POND EAST PROJECT FEATURE	CHECK LIST  DATE JULY 7,1978  NAME	
AREA EVALUATED	CONDITION	
OUTLET WORKS - OUTLET STRUCTURE AND OUTLET CHANNEL		
General Condition of Concrete		
Rust or Staining		
Spalling		
Erosion or Cavitation		
Visible Reinforcing		
Any Seepage or Efflorescence	NoT	and the second
Condition at Joints	APPLICABLE	
Drain holes		
Channel		
Loose Rock or Trees Overhanging Channel		
Condition of Discharge Channel	·	
	·	
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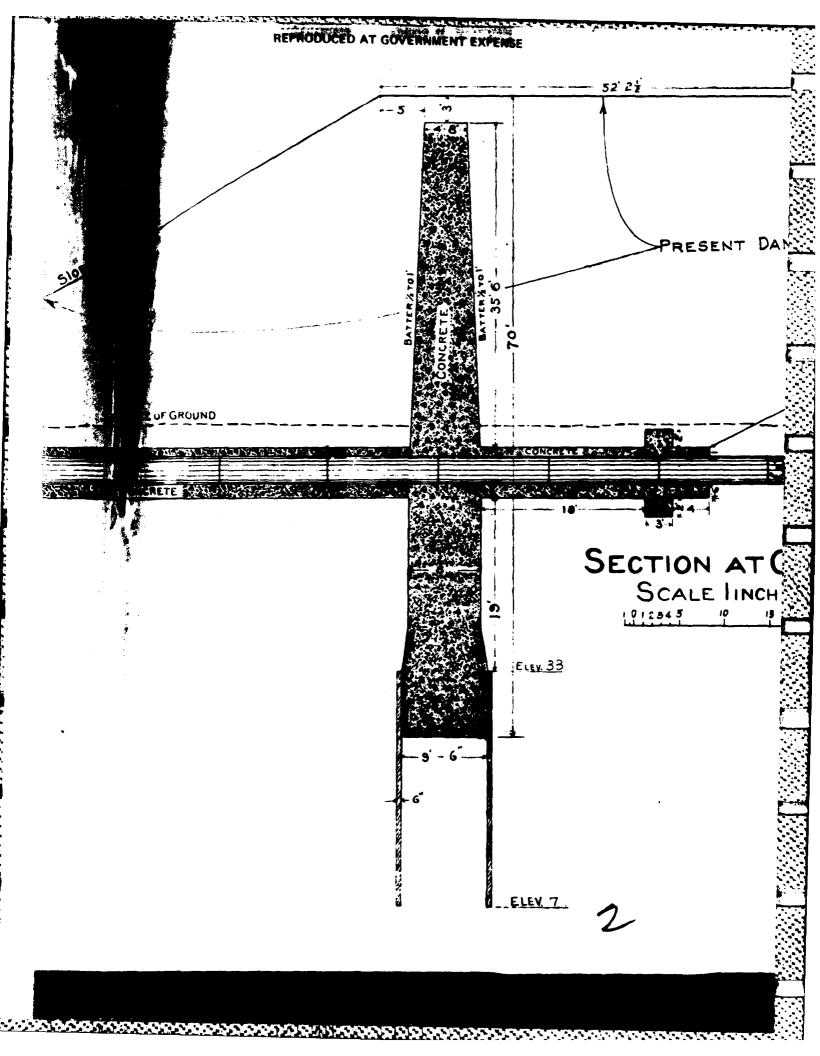
INSPECTION	CHECK LIST	
JECT WALDEN PONO EAST	DATE JULY 7, 1970	8
JECT FEATURE	NAME	
AREA EVALUATED	CONDITION	·
LET WORKS - SERVICE BRIDGE		
Super Structure		
Bearings	·	
Anchor Bolts		
Bridge Seat		
Longitudinal Members		
Under Side of Deck		
Secondary Bracing	NoT	•
Deck	APPLICABLE	
Drainage System		
Railings		
Expansion Joints		
Paint		
Abutment & Piers		
General Condition of Concrete		
Alignment of Abutment		
Approach to Bridge		
Condition of Seat & Backwall	·	
	·	•
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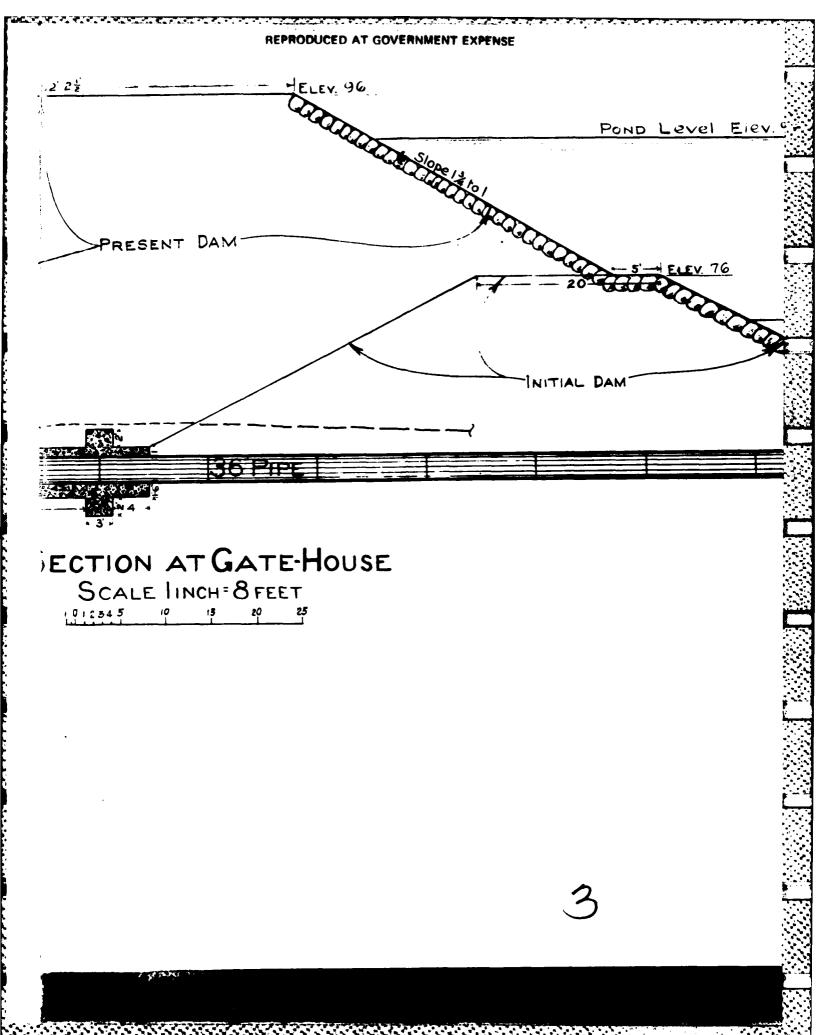
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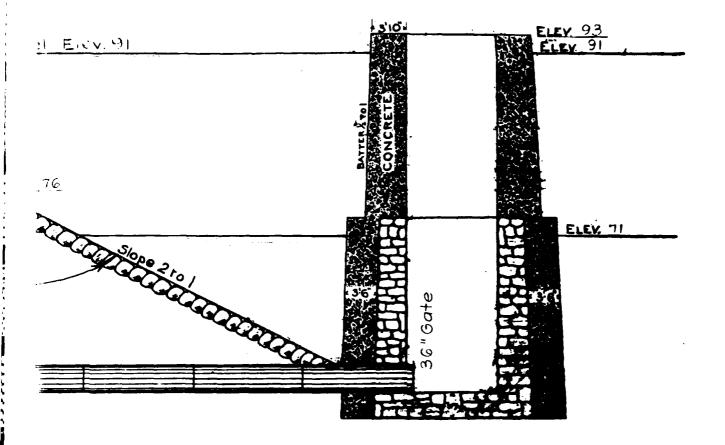




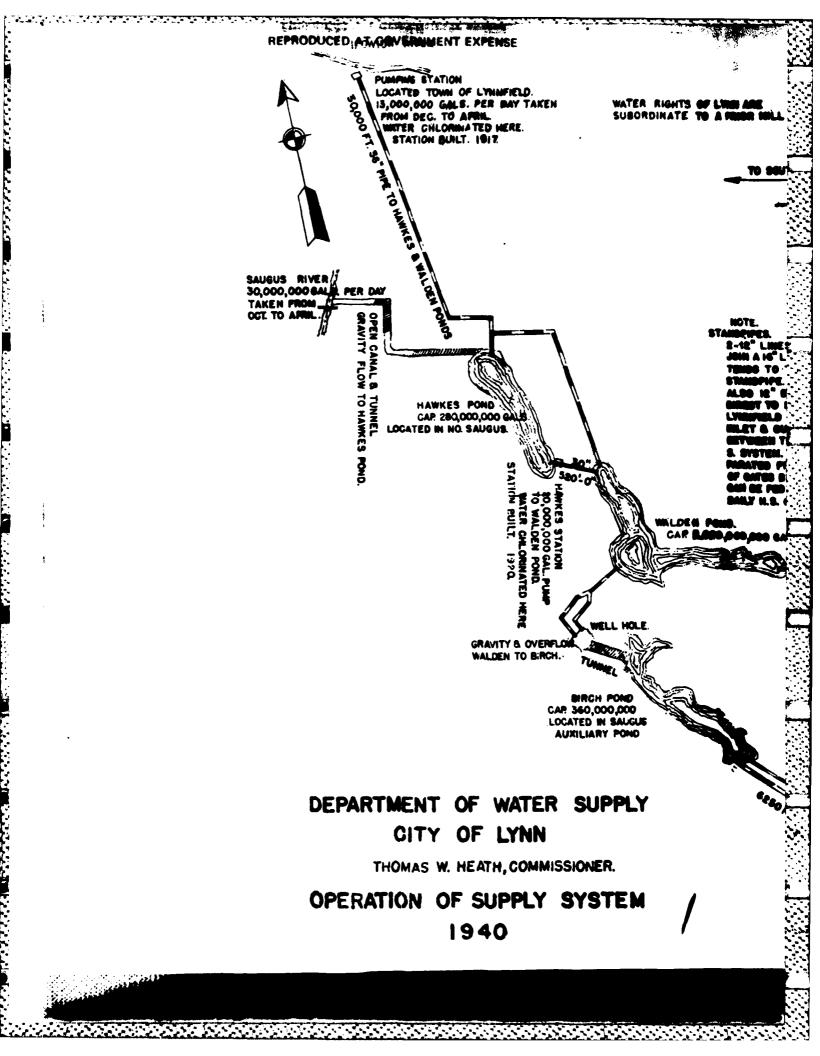


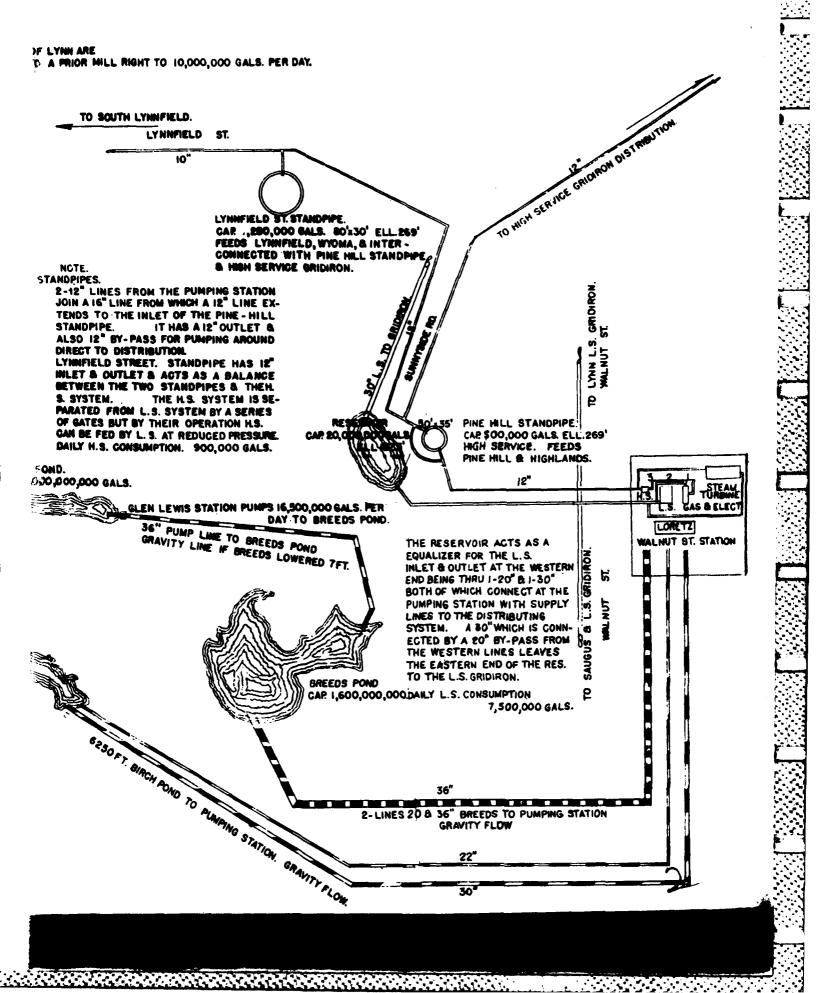


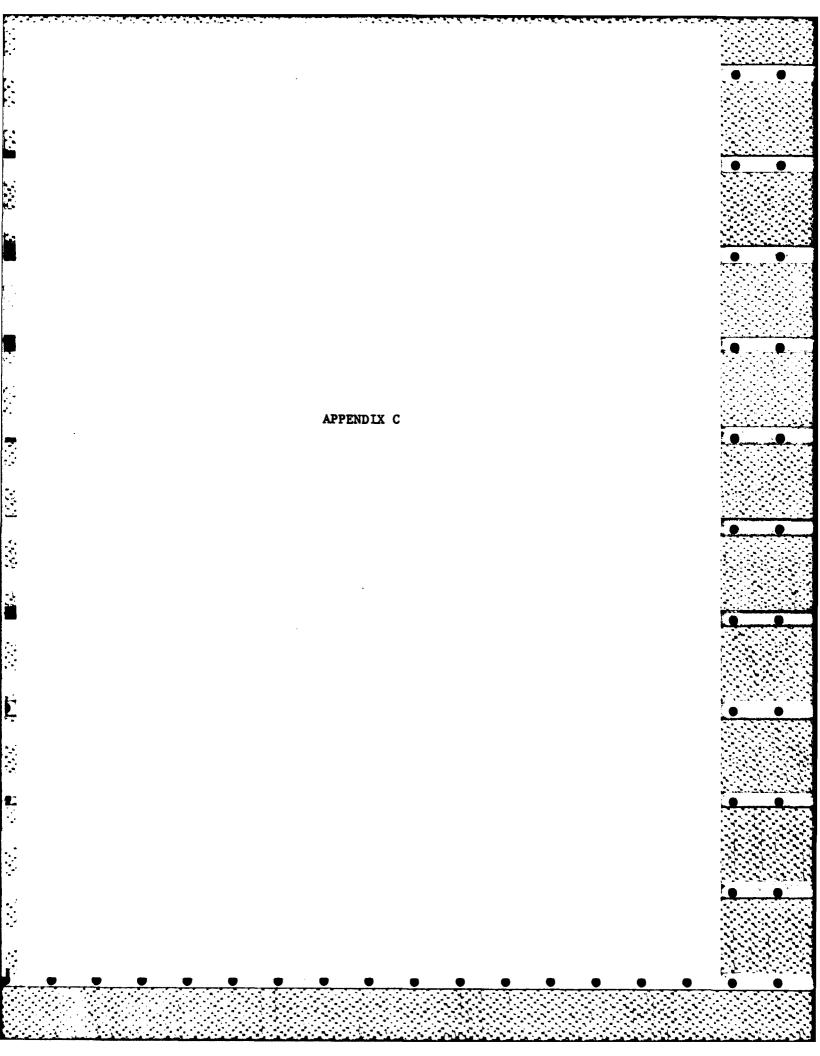
## REPRODUCED AT GOVERNMENT EXPENSE

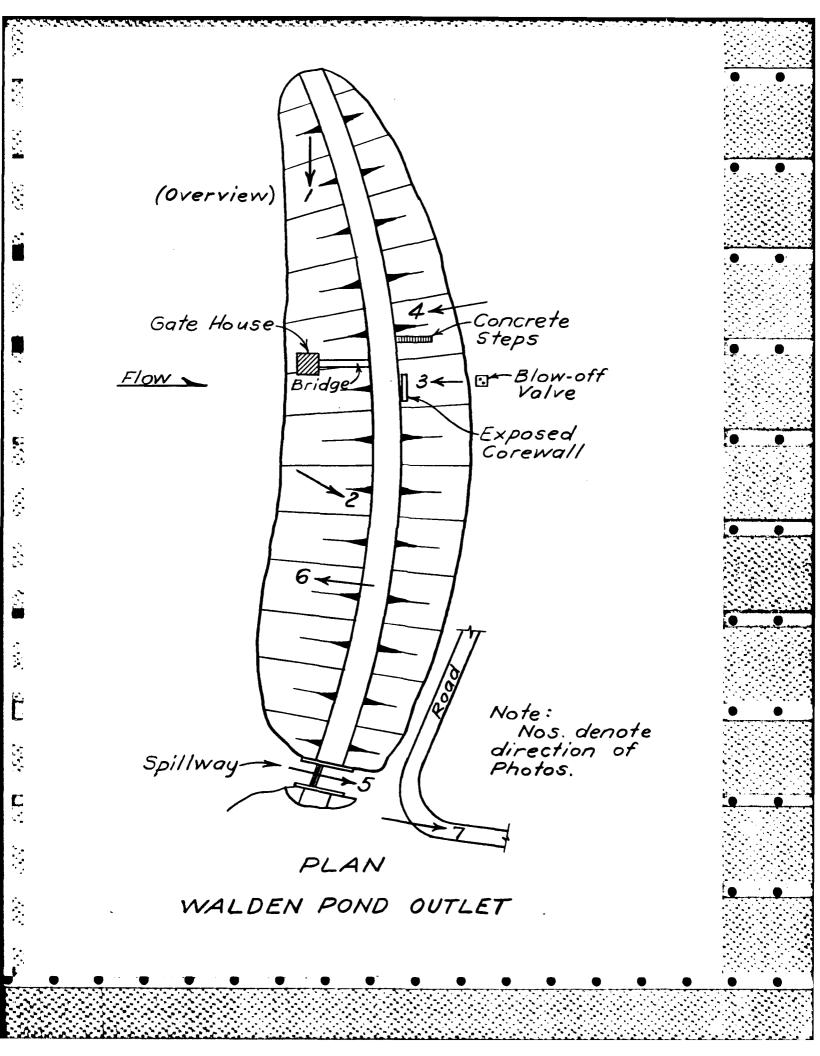


WALDEN POND



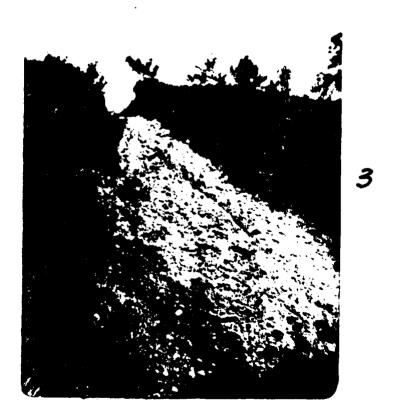








Overgrown Upstream Face



Eroded Path on Downstream Face



Exposed Corewall



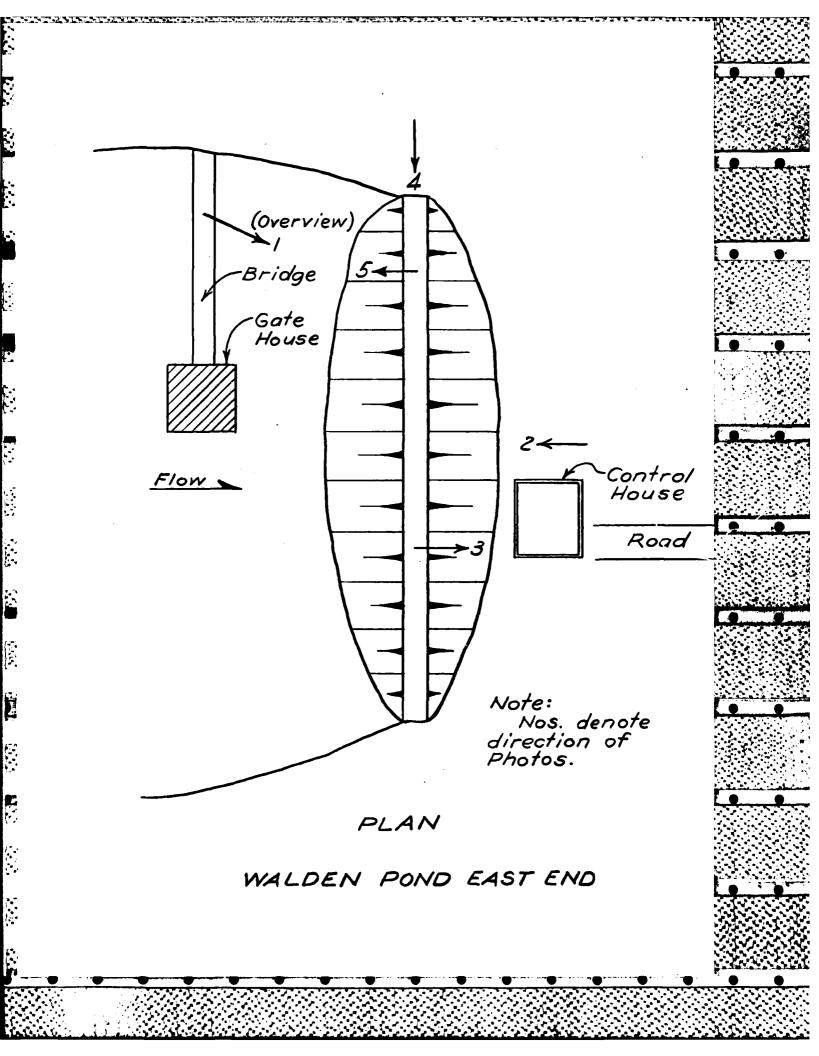
Heavily Overgrown and Strewn Spillway



General View of Reservoir



Spillway Discharge into Road Area





Downstream Face of Dam



Looking Downstream of Dam

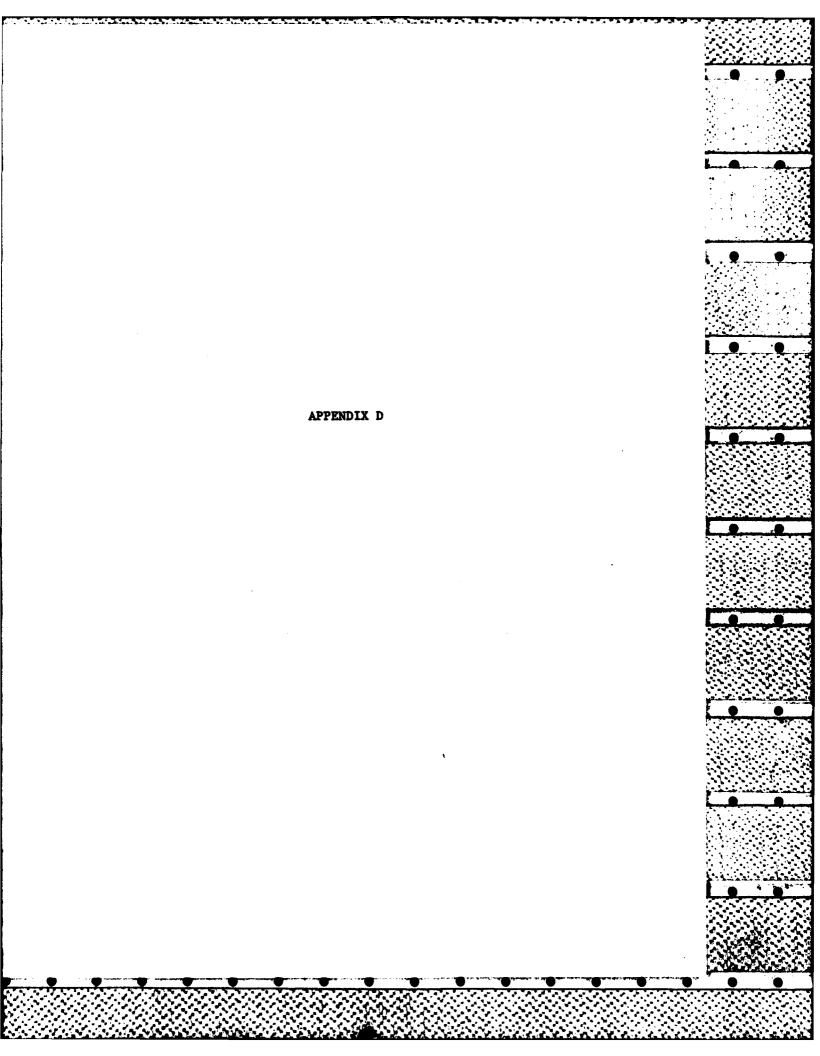


Looking Across Crest of Dam from Left Bank

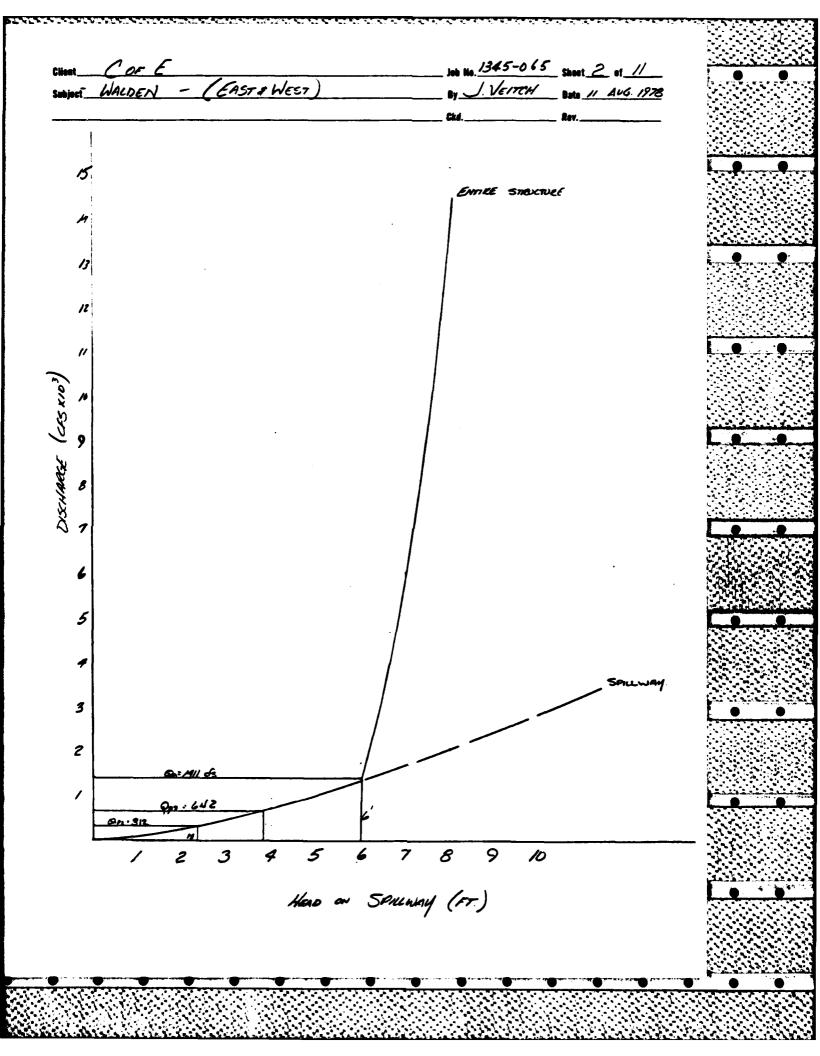


Looking West from Dam

WALDEN POND EAST END

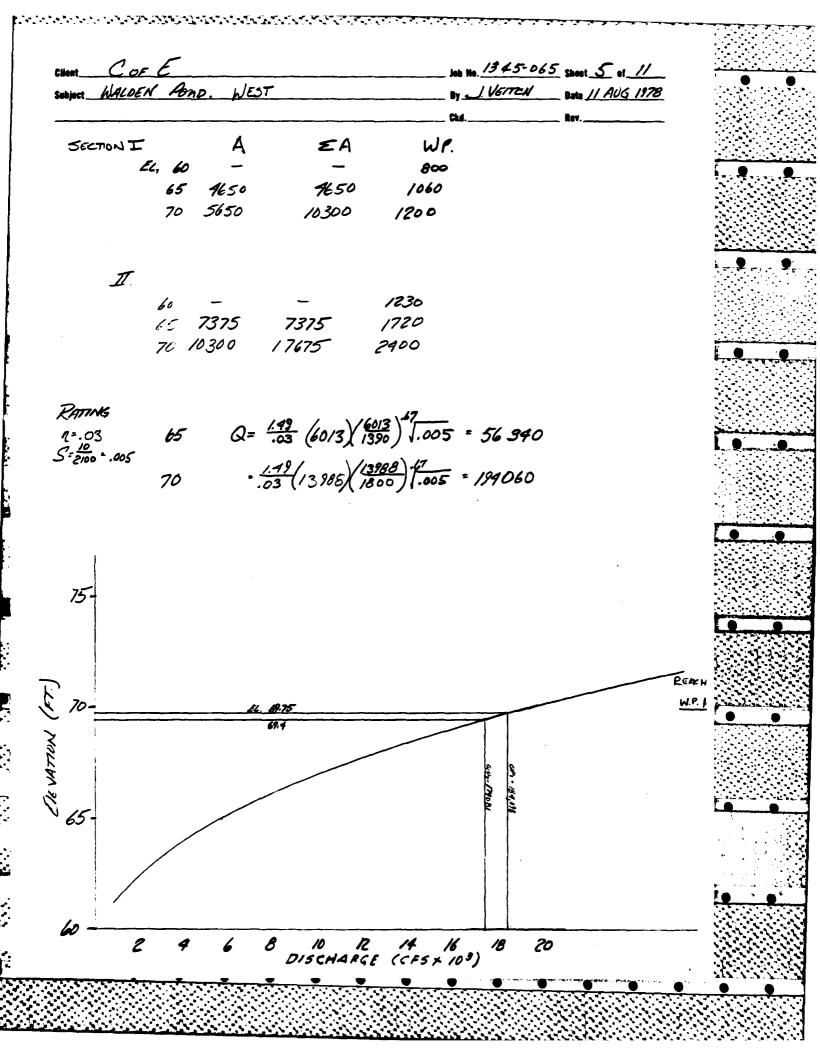


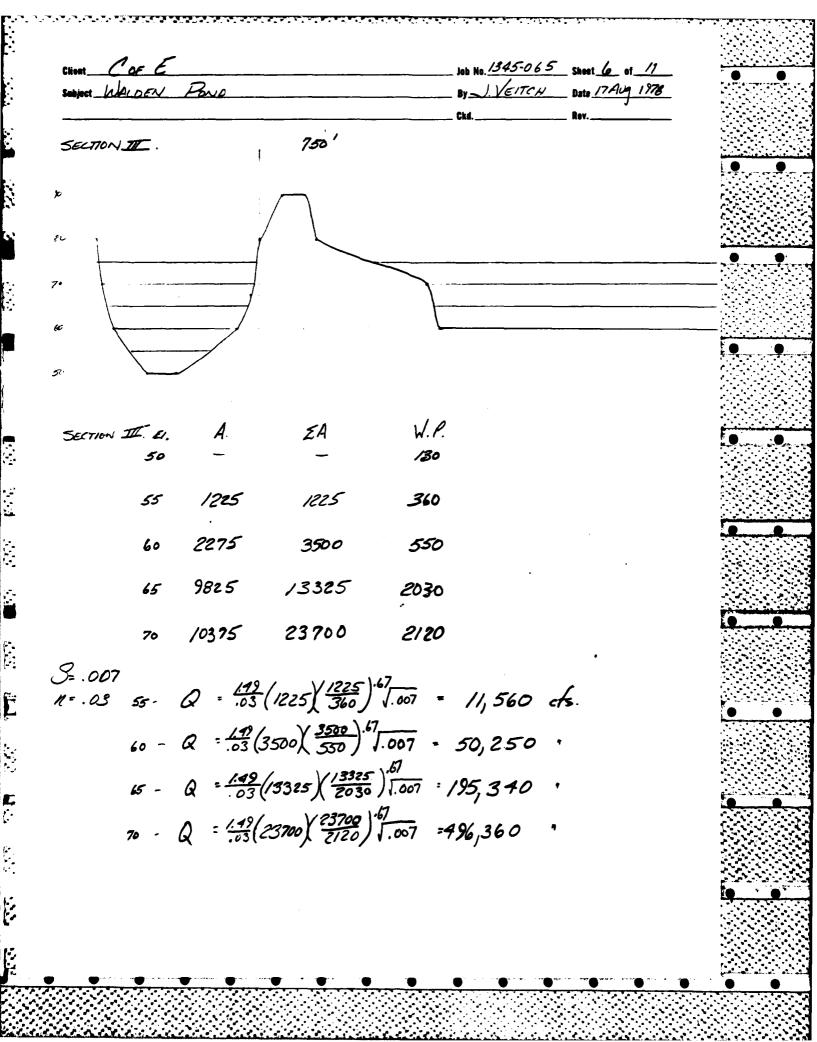
Cor E		25-065 Sheet / of //	
WALDEN POND (EAST & WEST)	By 1/E	17CH Date // AUG. 1978	
· · · · · · · · · · · · · · · · · · ·	Ckd	Rev	
_			
= 1911 ds			
= 1911 ds :e Aein: 1.75mi² = 1120 Ac			
YWE AREA · 230 AC			
WE RATTING.		- <b>!</b>	•
L=200			
ST	EL. 102	yo = 10-12'	
e) C = 2.25	)	,	
OUTLET PIPE N	NEGLECTED.	ASSIMING = ELEVATION.	
	(	M33 M 11140 - EEL 11111 - 1111	
L= 1750	)		
7. 2.25=C	22. 97	y = 35°	
(-3.0 )6 _	91		
<i>3</i> 0	•		
GS QUAD SHEETS SHOW DAM to be	< IND PROPERTY	al Paral	
GS WORD SHEETS SHOW DAIT TO C	- 100 ECEVATION	N BUT ON	
PECTION DAM & DIKE APPEAR.	2	N BUI ON	
_	2	N BUI ON	
CECTION DAM & DIKE APPEAR. TING CURVE SURCHARGE CALCULA	> 100 ' TIED ON TOTAL	BUT INDIVIDUAL	
PECTION DAM & DIKE APPEAR • TING <u>CURVE</u> SURCHARGE CALCULA DISCHARGES CONSIDER	- 100' TIED ON TOTAL SED TO APPROPRIS	BUT INDIVIOUAL TE CHANNELS.	
PECTION DAM & DIKE APPETIR • TING <u>CURVE</u> SURCHARGE VALCULA DISCHARGES COMSIDERS FAILURE CALCULATED	- 100' TIED ON TOTAL SED TO APPROPRIS	BUT INDIVIOUAL TE CHANNELS.	
PECTION DAM & DIKE APPEAR • TING <u>CURVE</u> SURCHARGE CALCULA DISCHARGES CONSIDER	- 100' TIED ON TOTAL SED TO APPROPRIS	BUT INDIVIOUAL TE CHANNELS.	
PECTION DAM & DIKE APPEAR. TING <u>CURVE</u> SURCHARGE CALCULA DISCHARGES CONSIDERS FAILURE CALCULATED WEST CALCULATED.	- 100' TIED ON TOTAL SED TO APPROPRIS	BUT INDIVIOUAL TIE CHANNELS.	
PECTION DAM & DIKE APPEAR.  TING CURVE SURCHARGE CALCULA  DISCHARGES COMSIDERS  FAILURE CALCULATED  WEST CALCULATED.	- 100' THED ON TOTAL BED TO APPROPRIA I.: EAST FAI	BUT INDIVIDUAL TE CHANNELS. YLURE AND II:	
PECTION DAM & DIKE APPEAR. TING <u>CURVE</u> SURCHARGE CALCULA DISCHARGES CONSIDERS FAILURE CALCULATED WEST CALCULATED.	- 100' TIED ON TOTAL SED TO APPROPRIS	BUT INDIVIOUAL TIE CHANNELS.	
PECTION DAM & DIKE APPEAR.  TING CURVE SUPERARGE CALCULA  DISCHARGES COMSIDERS  FAILURE CALCULATED  WEST CALCULATED.  4. Ques	- 100' THED ON TOTAL BED TO APPROPRIA I.: EAST FAI	BUT INDIVIOUAL  OF TOTAL	
PECTION DAM & DIKE APPEAR.  TING CURVE SURCHARGE CALCULA  DISCHARGES COMSIDERS  FAILURE CALCULATED  WEST CALCULATED.  4. Quint  2 255	- 100' THED ON TOTAL BED TO APPROPRIA I.: EAST FAI	BUT INDIVIOUAL  OF THE CHANNELS.  OF THE CHANNELS.  OF THE CHANNELS.  OF THE CHANNELS.  255	
PECTION DAM & DIKE APPENR -  TING CURVE SURCHARGE CALCULA  DISCHARGES CONSIDERS  FAILURE CALCULATED  WEST CALCULATED.  4. Ques  2. 255  4. 720	- 100' THED ON TOTAL BED TO APPROPRIA I.: EAST FAI	BUT INDIVIOUAL  THE CHANNELS.  LURE AND IT:  255  720	
PECTION DAM & DIKE APPEAR.  TING CURVE SUCCHARGE CALCULA  DISCHARGES COMSIDERS  FAILURE CALCULATED  WEST CALCULATED.  4. Quint  2. 255  4. 720  6. 1323	- 100' TOTAL TED TO APPROPRIA L: EAST FAI	BUT INDIVIOUAL  THE CHANNELS.  VLURE AND IT:  255 720 1323	
PECTION DAM & DIKE APPENR -  TING CURVE SURCHARGE CALCULA  DISCHARGES CONSIDERS  FAILURE CALCULATED  WEST CALCULATED.  4. Ques  2. 255  4. 720	- 100' THED ON TOTAL BED TO APPROPRIA I.: EAST FAI	BUT INDIVIOUAL  THE CHANNELS.  LURE AND IT:  255  720	
PECTION DAM & DIKE APPEAR.  TING CURVE SUCCHARGE CALCULA  DISCHARGES COMSIDERS  FAILURE CALCULATED  WEST CALCULATED.  4. Quint  2. 255  4. 720  6. 1323	- 100' TOTAL TED TO APPROPRIA L: EAST FAI	BUT INDIVIOUAL  THE CHANNELS.  VLURE AND IT:  255 720 1323	
PECTION DAM & DIKE APPEAR.  TING CURVE SUCCHARGE CALCULA  DISCHARGES COMSIDERS  FAILURE CALCULATED  WEST CALCULATED.  4. Quint  2. 255  4. 720  6. 1323	- 100' TOTAL TED TO APPROPRIA L: EAST FAI	BUT INDIVIOUAL  THE CHANNELS.  VLURE AND IT:  255 720 1323	
PECTION DAM & DIKE APPEAR.  TING CURVE SUCCHARGE CALCULA  DISCHARGES COMSIDERS  FAILURE CALCULATED  WEST CALCULATED.  4. Quint  2. 255  4. 720  6. 1323	- 100' TOTAL TED TO APPROPRIA L: EAST FAI	BUT INDIVIOUAL  THE CHANNELS.  VLURE AND IT:  255 720 1323	
PECTION DAM & DIKE APPEAR.  TING CURVE SUCCHARGE CALCULA  DISCHARGES COMSIDERS  FAILURE CALCULATED  WEST CALCULATED.  4. Quint  2. 255  4. 720  6. 1323	- 100' TOTAL TED TO APPROPRIA L: EAST FAI	BUT INDIVIOUAL  THE CHANNELS.  VLURE AND IT:  255 720 1323	
PECTION DAM & DIKE APPEAR.  TING CURVE SUCCHARGE CALCULA  DISCHARGES COMSIDERS  FAILURE CALCULATED  WEST CALCULATED.  4. Quint  2. 255  4. 720  6. 1323	- 100' TOTAL TED TO APPROPRIA L: EAST FAI	BUT INDIVIOUAL  THE CHANNELS.  VLURE AND IT:  255 720 1323	
PECTION DAM & DIKE APPEAR.  TING CURVE SUCCHARGE CALCULA  DISCHARGES COMSIDERS  FAILURE CALCULATED  WEST CALCULATED.  4. Quint  2. 255  4. 720  6. 1323	- 100' TOTAL TED TO APPROPRIA L: EAST FAI	BUT INDIVIOUAL  THE CHANNELS.  VLURE AND IT:  255 720 1323	

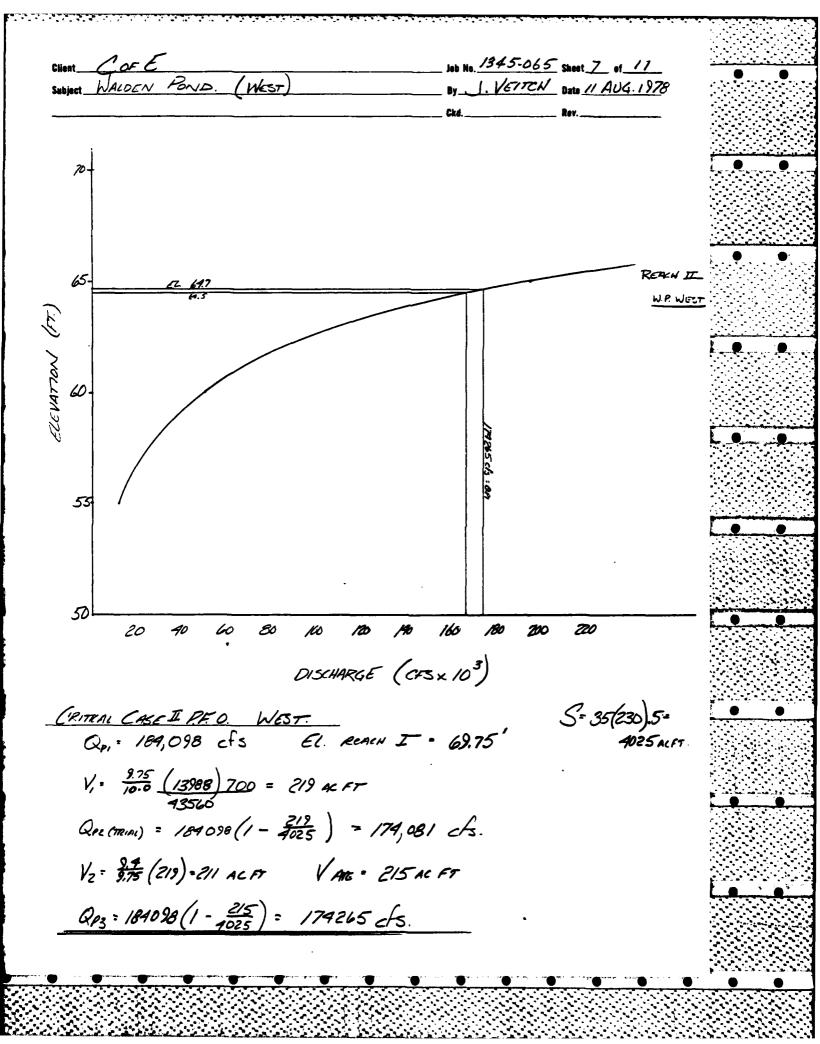


	13/5-06		
Client Car E	_	Sheet 3 et //	
Subject WALDEN POND (EASTS WEST)	By J. VC. ICA	Date // AUG. 1978	
		_ REV	
Q1 - 1411 c/s 5-6.0' (F)	OW CONTAINED W	DITANN SPILLWAY).	
(6.0 X12 Y230)			
STOR, = (6.0×12×230) = 14.8 "			
Qp= 1411(1- 195) - 312 cfs Sz= 2.4' S	70R2. 2.4(12/230)	" 5.9" Stora 1435	
QR3 = 1411 (1-10.35) = 642 cfs. :- TEST FLOOD CASE I : INVESTIGATION SALLWAY ON WEST OUTLET.			
:- TEST FLOOD CASE I : INVESTIGATION	OF 642	cts through	
SALLWAY ON WEST OUTLET.			
			•
PEAK FAILURE CASE I : FAILURE &	AST DIKE	P.F.O. Exclusively (Filly)	
		-#	
PEAR FAILURE CASE II: " WE	ST UAIT. # 1	CAPACITY OF SPALWAY)	
		<del></del>	
P.FO CASE I EAST DIKE	6=12'	_	
	b = 12 ' b = .3(200) = 60 '	,	
Qp, = B (60) (32.2) 12)1.5			
		•	
= 4,194 cfs.			
PFO CASE TT WEST DAM	1 = 35		
West Survey Surv	16 = 35 1 <sub>6</sub> = .3 (1750) = 58	25'	
,			
Qp, = \$25 (525) (32.2) (35).3			
	<b>6</b> -4	•	
· 182,775 ds. + 64.	2 cfs = 18	4000 ds.	

COFÉ WALDEN POND - (WEST)		5 Sheet <u>4</u> of <u>11</u> Data 11 AUG-1978	•
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Chd	Rev	
MICAL CASE P.F.O. CASE II.			
,			
H Z DAM			
SECTION I BASE OF DAM			•
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SECTION IL 700			
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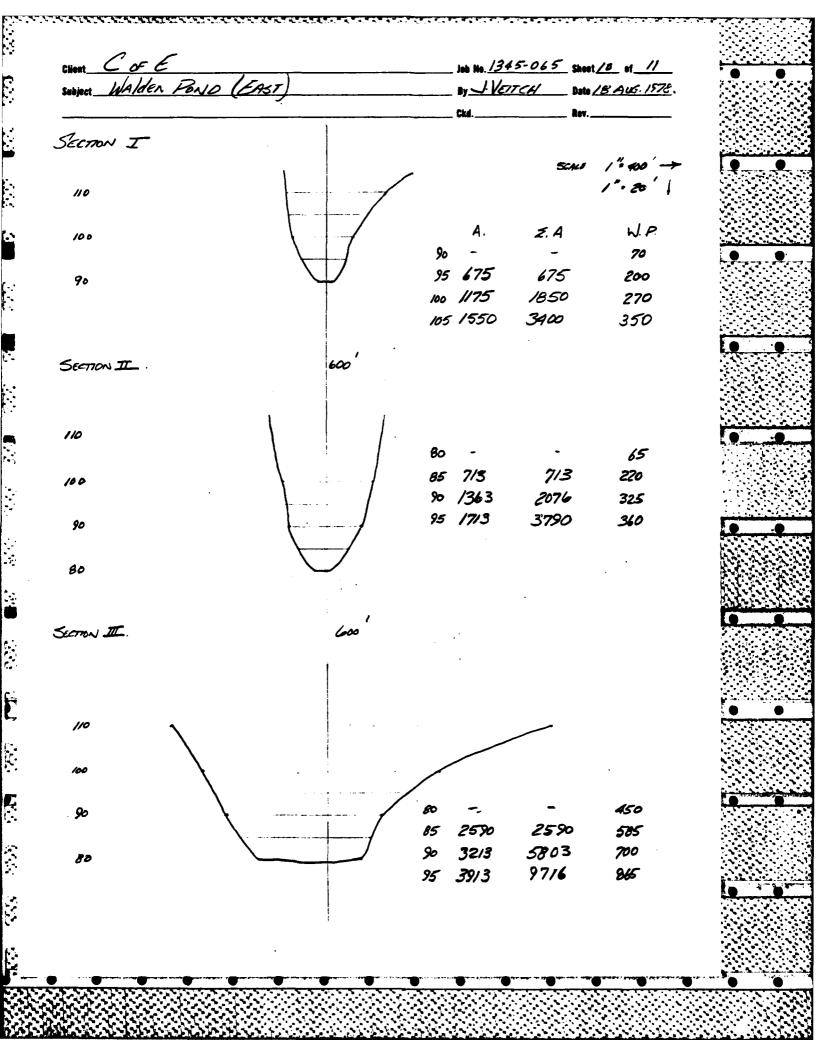


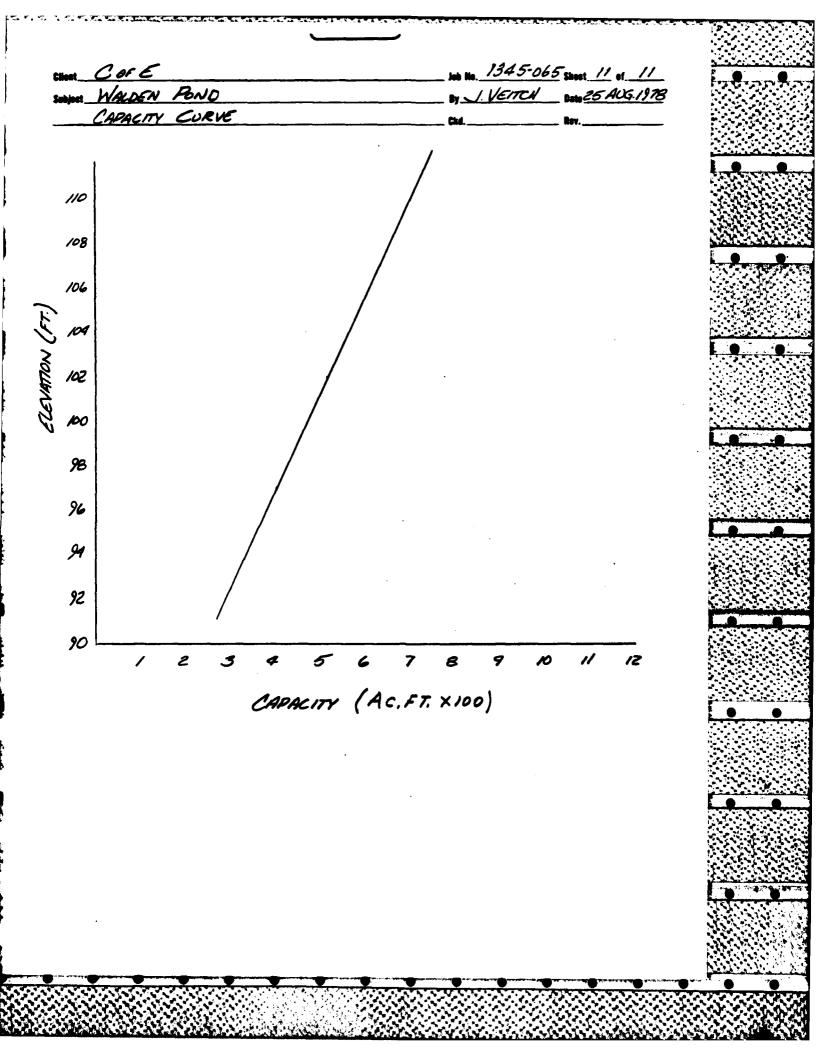


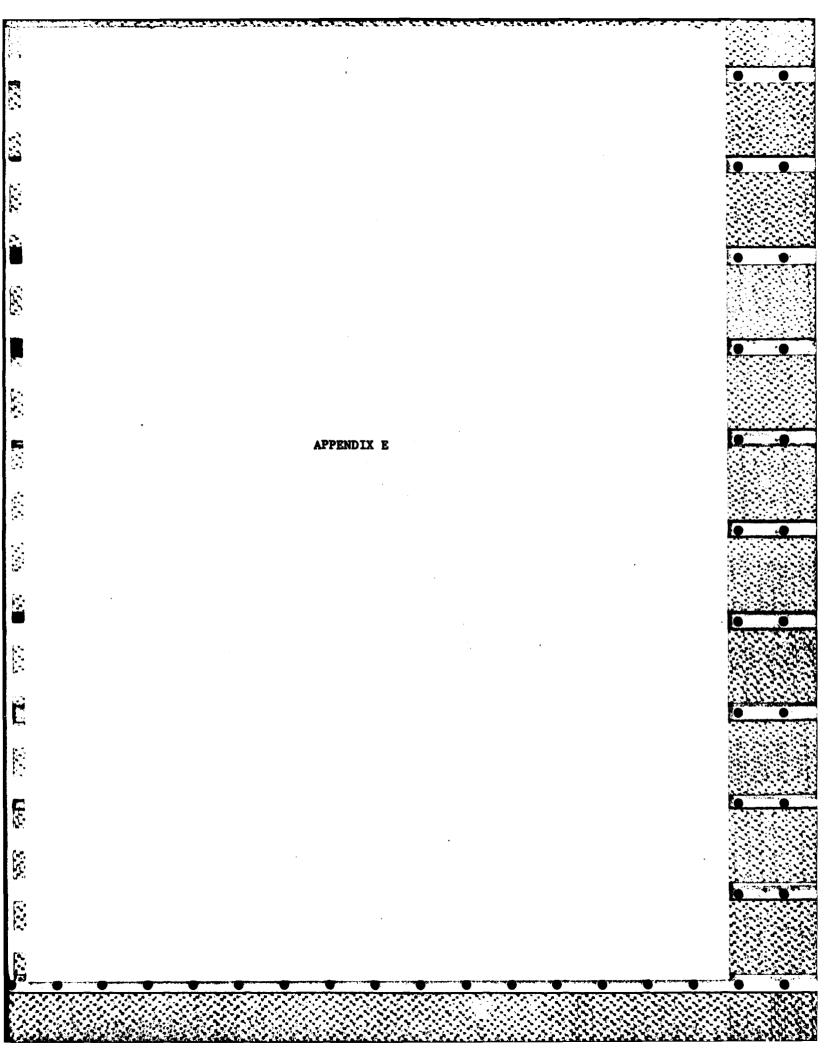
Client	COFE	Job No. 1345-06	<u> </u>	
Subject_	WALDEN POND ( WEST)	By J VETTCH	Date /E AUG. 1978	
<del></del>		Ckd	Rev	
REACH	"2 Qp = 174,265 c/s.	EL = 64.7		•
	Vi = 19.7 (10350) 750) = 175 A	C FT.		
	43560			
	( 175		<del>-</del> ** . • • •	•
	QP2 (TRIPL) = 174265 (1-4025) = 10	66,688 CFS		
	Vz = 14.5 (175) = 173 ACFT	1/ = 170 11 5=		
	V2 - 19.7 (173) - 173 XEFT	VAVE, 177 AC.F.	A Property of the Control of the Con	
	Qpg = 174 265 (1- 174) = 16	56.731 cfs		•
	4023 / 10	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		
	_			•
	I. Q= 4199 EC. = 84	15 REALN I	<del></del>	
	1 45/ V \ 2 -	•	/ 20.5	
	V, = \frac{4.5}{5} (1650)(600) = 20.5 AC FT	CIPZ(TRIAL) 4/94(	1- Ages 5 41/2	•
	NEGLECTING STORAGE REMEN # I	<del>,                                     </del>		
	NEGLECITAG STOCAGE REACH I	•		
	Q1= 4194 EC. = 83.0	REACH #2	i de la companya de l	-
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			10 Min 11 Min 11 Min 11 Min	
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Job No. 1345-065 Sheet 9 of 11 Subject WALDEN (EAST) VETTIN Bate 18 AUG. 1978 I S= .017 REACH RATING CURVES K=.03 85 Q= 1.49 (356) (356) (.017) = 5064 50 Q = 198 (1038) (1038) (1017) 4 · 20400 95 Q · 1.99 (2233) 280) (017) · 58/20 S=.00Z n = .03 $Q = \frac{1.49}{03} \left( \frac{1650}{403} \right) \left( \frac{.67}{.002} \right)^{1/2} = 9425 \text{ cfs}$ Q. \(\frac{149}{05}\)(3940)\(\frac{3940}{573}\)\(\frac{1}{002}\)\(\frac{1}{2} - 34300\) 95  $Q = \frac{1.49}{.03} (6753) \left( \frac{6753}{613} \right)^{.67} (.002)^{1/2} = 74860$  " WALDEN EAST 95. DISCHARGE (CFSX 103)







## END

## FILMED

7-85

DTIC